# **Mechanics of Thin-walled Structures**

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#### 1. Basic concepts of mechanics of structures

- a. Stress
- b. Strain
- c. Moment of inertia
  - i. First (static) moment of area (static)
  - ii. Second moment of area (inertia)

2. Thin-walled structures introduction

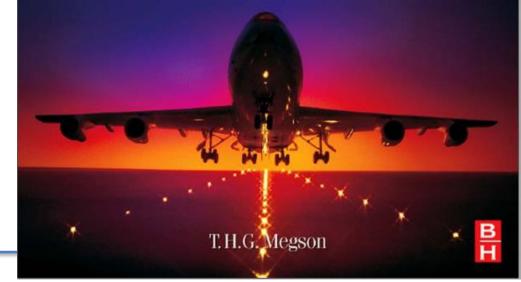
- 3. Beams
  - a. Bending of beams
    - i. Shear centre
    - ii. Open section beams
      - First (static) moment of inertia approach
      - Function approach
    - iii. Closed section beams
  - b. Torsion of beams
    - i. Free torsion
    - ii. Constrained torsion

- 4. Plates and shells (2D structures)
- 5. Buckling
  - a. Analytical approach
  - b. Energy approach
  - c. Buckling of columns
  - d. Buckling of plates
  - e. Buckling of shells



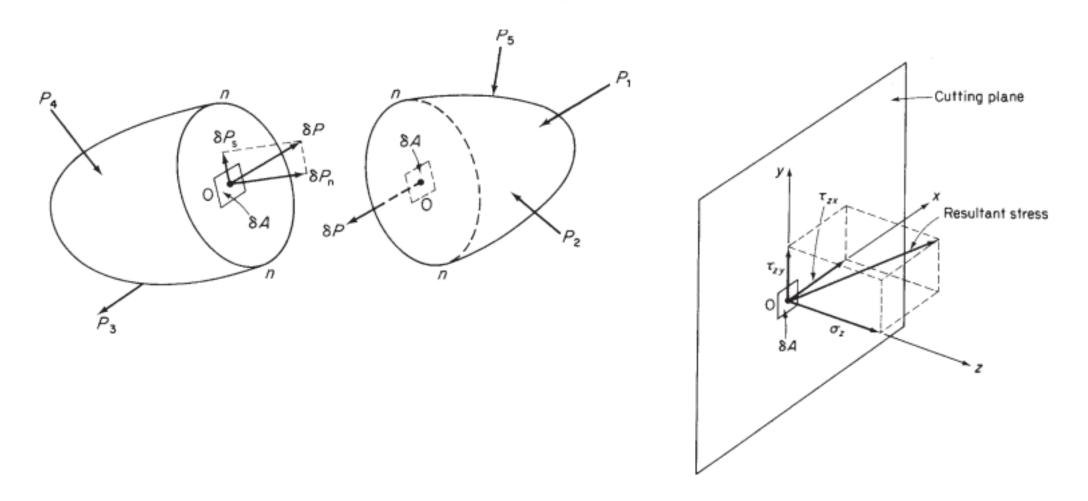


Fourth Edition



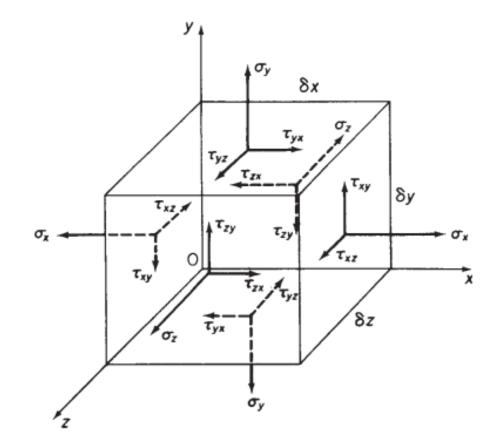
# Stress, Strain, Hooke`s law moments of Area

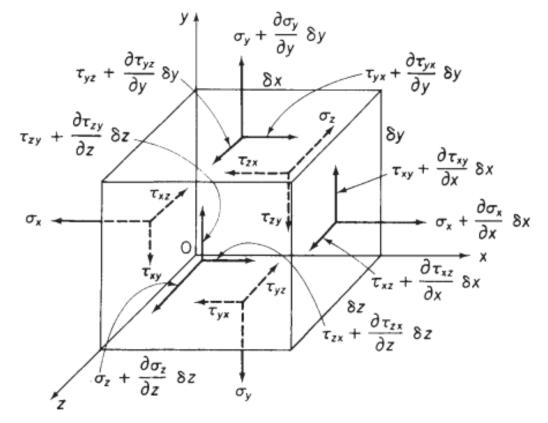
**State of stress** 



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### **State of stress**

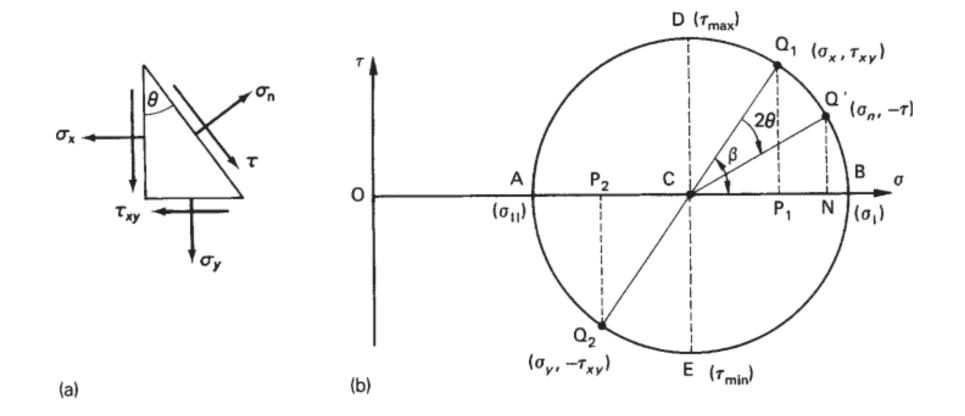
$$\frac{\partial \sigma_x}{\partial x} + \frac{\partial \tau_{xy}}{\partial y} + \frac{\partial \tau_{xz}}{\partial z} + X = 0$$

$$\frac{\partial \sigma_y}{\partial y} + \frac{\partial \tau_{yx}}{\partial x} + \frac{\partial \tau_{yz}}{\partial z} + Y = 0$$

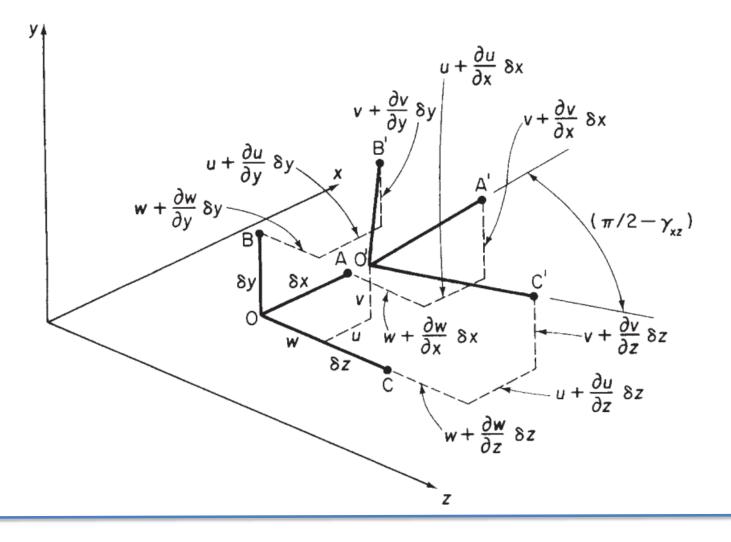
$$\frac{\partial \sigma_z}{\partial z} + \frac{\partial \tau_{zx}}{\partial x} + \frac{\partial \tau_{zy}}{\partial y} + Z = 0$$

$$\frac{\partial \sigma_x}{\partial x} + \frac{\partial \tau_{xy}}{\partial y} + X = 0$$
$$\frac{\partial \sigma_y}{\partial y} + \frac{\partial \tau_{yx}}{\partial x} + Y = 0$$





#### **State of strain**



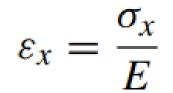
#### **State of strain**

$$\left. \begin{array}{l} \varepsilon_{x} = \frac{\partial u}{\partial x} \\ \varepsilon_{y} = \frac{\partial v}{\partial y} \\ \varepsilon_{z} = \frac{\partial w}{\partial z} \end{array} \right\} \qquad \qquad \begin{array}{l} \gamma_{xz} = \frac{\partial w}{\partial x} + \frac{\partial u}{\partial z} \\ \gamma_{xy} = \frac{\partial v}{\partial x} + \frac{\partial u}{\partial y} \\ \gamma_{yz} = \frac{\partial w}{\partial y} + \frac{\partial v}{\partial z} \end{array}$$

 $\sim$ 

$$\varepsilon = \lim_{L \to 0} \frac{\Delta L}{L}$$

# Stress – strain relations 1-D case



$$\varepsilon_y = -\nu \frac{\sigma_x}{E} \quad \varepsilon_z = -\nu \frac{\sigma_x}{E}$$

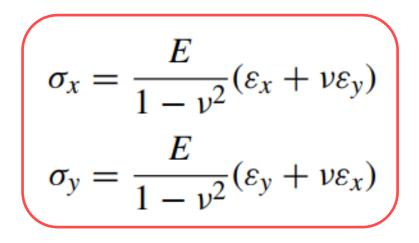
# Stress – strain relations 3-D case

$$\varepsilon_x = \frac{1}{E} [\sigma_x - \nu(\sigma_y + \sigma_z)]$$
$$\varepsilon_y = \frac{1}{E} [\sigma_y - \nu(\sigma_x + \sigma_z)]$$
$$\varepsilon_z = \frac{1}{E} [\sigma_z - \nu(\sigma_x + \sigma_y)]$$

$$\sigma_x = \frac{\nu E}{(1+\nu)(1-2\nu)}e + \frac{E}{(1+\nu)}\varepsilon_x$$
$$\sigma_y = \frac{\nu E}{(1+\nu)(1-2\nu)}e + \frac{E}{(1+\nu)}\varepsilon_y$$
$$\sigma_z = \frac{\nu E}{(1+\nu)(1-2\nu)}e + \frac{E}{(1+\nu)}\varepsilon_z$$

$$e = \varepsilon_x + \varepsilon_y + \varepsilon_z$$

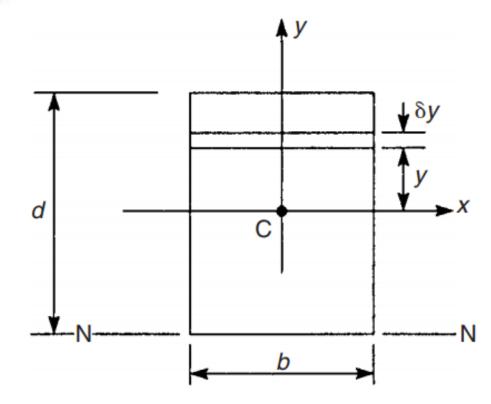
### Stress – strain relations 2-D case



$$\gamma = \tau/G$$

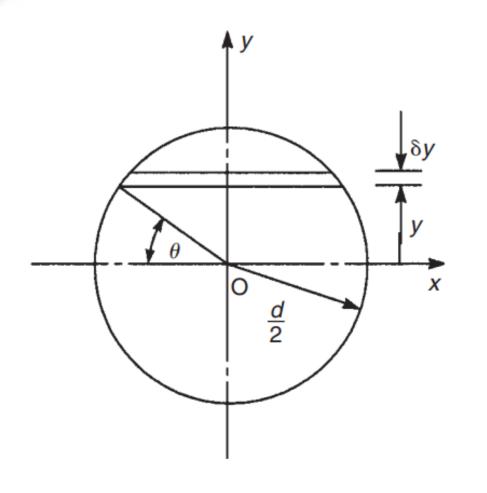
$$\gamma = \frac{2(1+\nu)}{E}\tau$$

## **Second moment of area**



$$I_{xx} = \int_{A} y^{2} dA = \int_{-d/2}^{d/2} by^{2} dy = b \left[ \frac{y^{3}}{3} \right]_{-d/2}^{d/2}$$
$$I_{xx} = \frac{bd^{3}}{12}$$

#### **Second moment of area**



$$I_{xx} = \int_A y^2 dA = \int_{-d/2}^{d/2} 2\left(\frac{d}{2}\cos\theta\right) y^2 dy$$

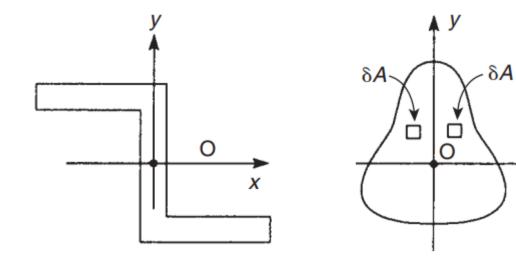
$$I_{xx} = \int_{-\pi/2}^{\pi/2} d\cos\theta \left(\frac{d}{2}\sin\theta\right)^2 \frac{d}{2}\cos\theta \,d\theta$$

$$I_{xx} = \frac{d^4}{8} \int_{-\pi/2}^{\pi/2} \cos^2\theta \, \sin^2\theta \, \mathrm{d}\theta$$

$$I_{xx} = \frac{\pi d^4}{64}$$

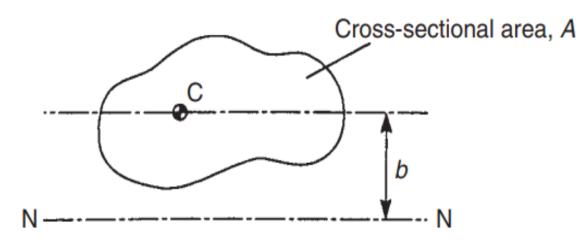
#### **Product second moment of area**

X



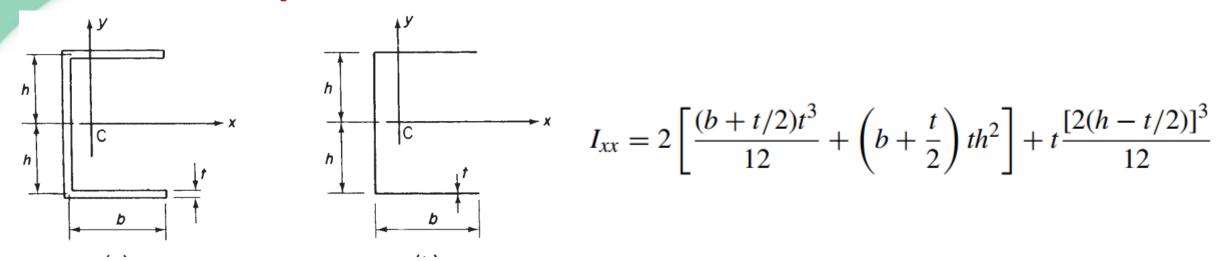
 $I_{xy} = \int_A xy \, \mathrm{d}A$ 

#### **Parallel axes theorem (Steiner principle)**



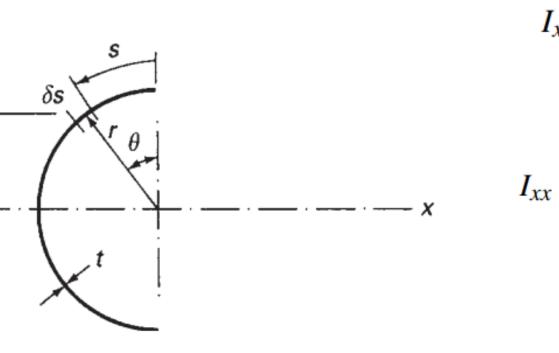
$$I_{\rm N} = I_{\rm C} + Ab^2$$

#### **Aproximations for Thin-Walled sections**



$$I_{xx} = 2\left[\frac{(b+t/2)t^3}{12} + \left(b + \frac{t}{2}\right)th^2\right] + \frac{t}{12}\left[(2)^3\left(h^3 - 3h^2\frac{t}{2} + 3h\frac{t^2}{4} - \frac{t^3}{8}\right)\right]$$
$$I_{xx} = 2bth^2 + t\frac{(2h)^3}{12}$$

# **Aproximations for Thin-Walled sections**



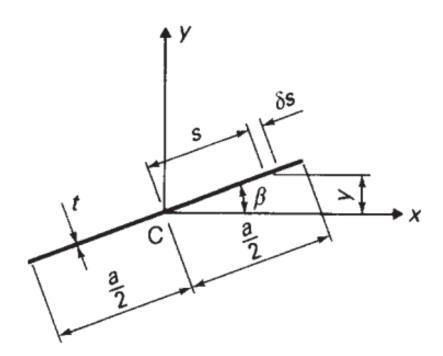
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$$I_{xx} = \int_0^{\pi r} ty^2 \,\mathrm{d}s$$

$$I_{xx} = \int_0^{\pi} t(r\cos\theta)^2 r\,\mathrm{d}\theta$$

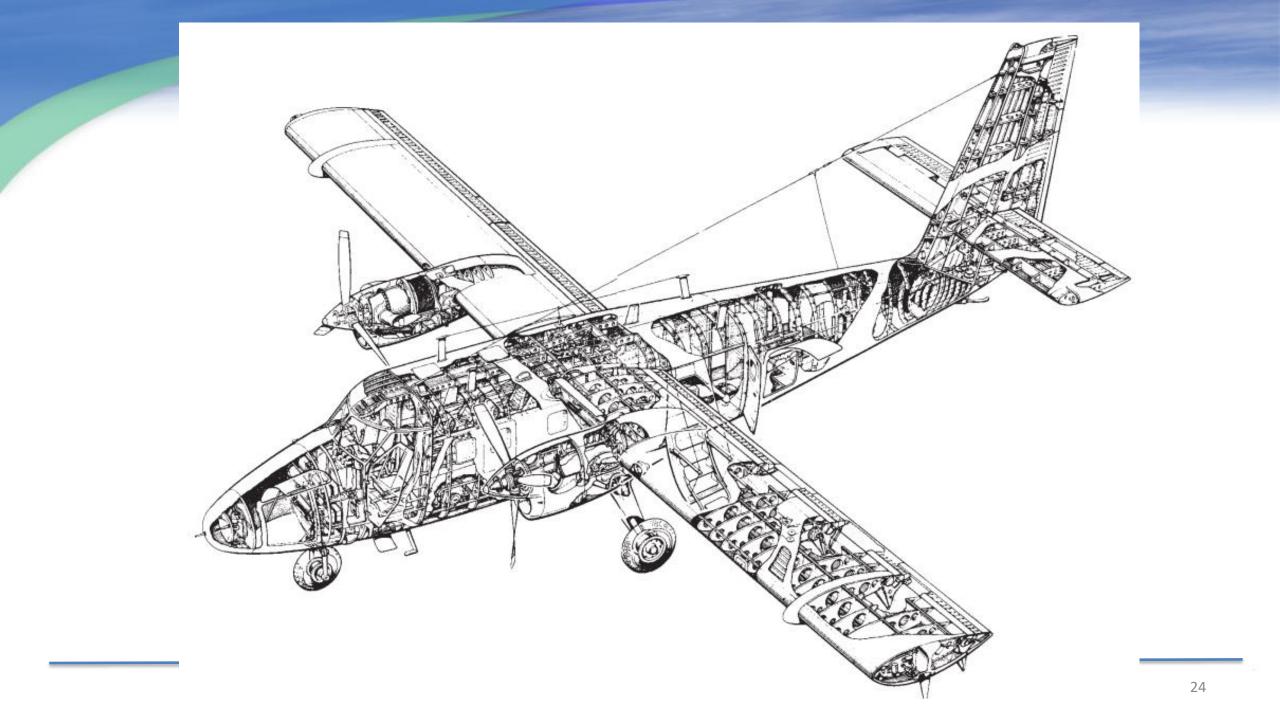
$$I_{xx} = \frac{\pi r^3 t}{2}$$

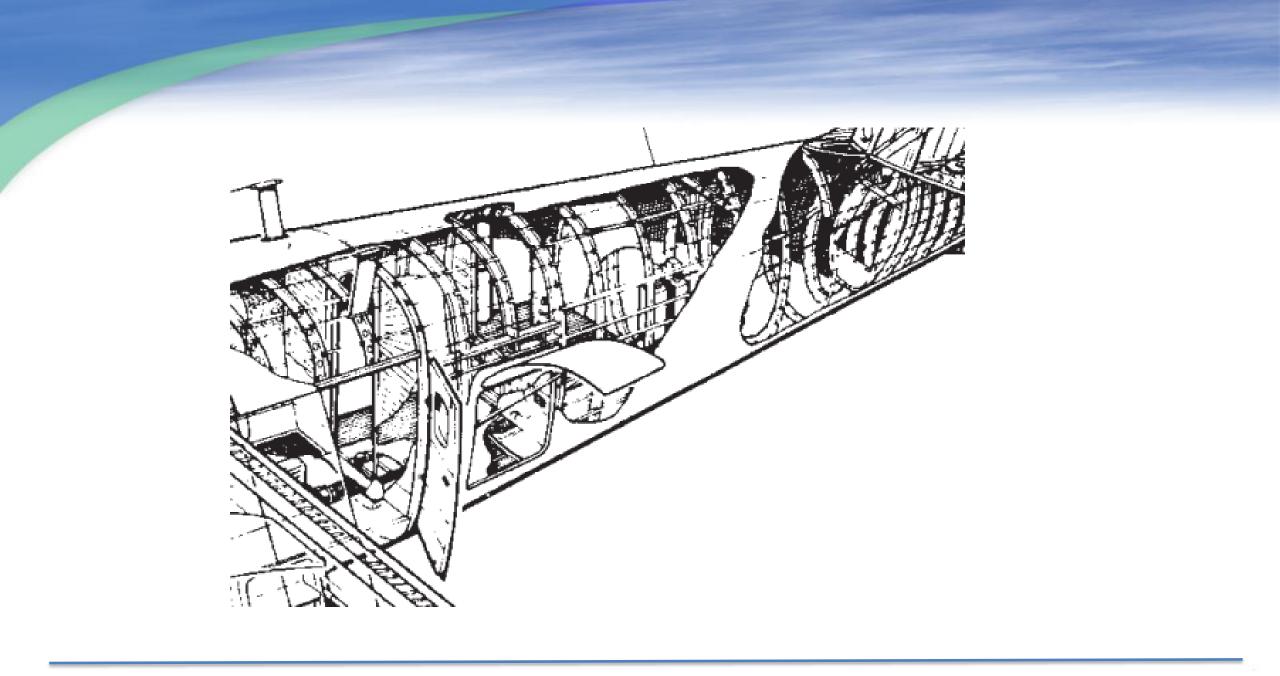
#### **Thin-Walled sections – inclined walls**



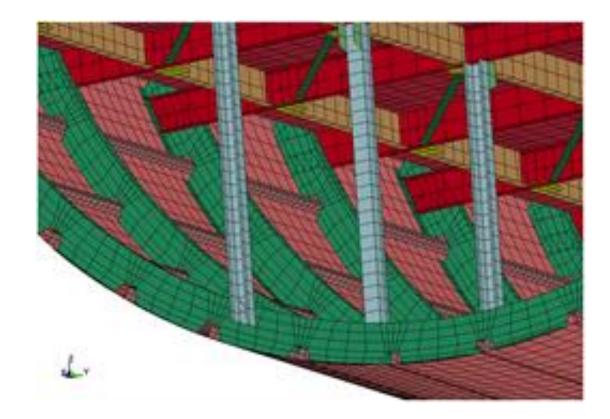
$$I_{xx} = 2 \int_0^{a/2} ty^2 \, ds = 2 \int_0^{a/2} t(s \sin \beta)^2 \, ds$$
$$I_{xx} = \frac{a^3 t \sin^2 \beta}{12}$$
$$I_{yy} = \frac{a^3 t \cos^2 \beta}{12}$$
$$I_{xy} = \frac{a^3 t \sin 2\beta}{24}$$

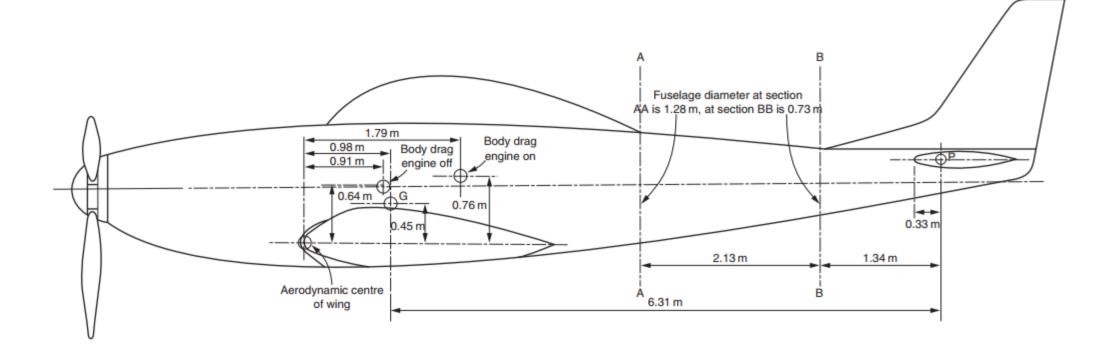
Thin-walled Structures Idealization and modelling

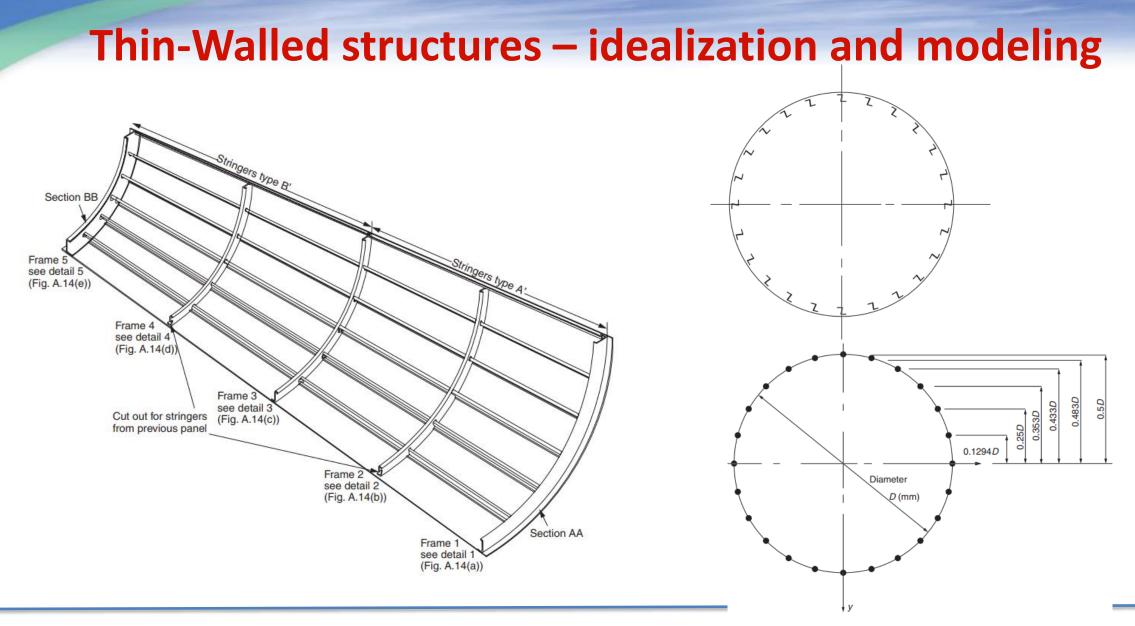


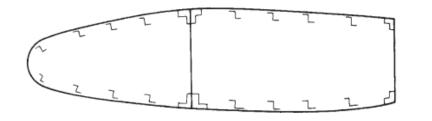


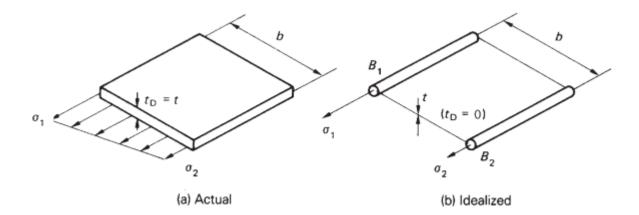


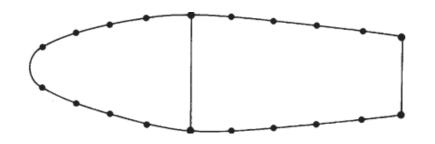


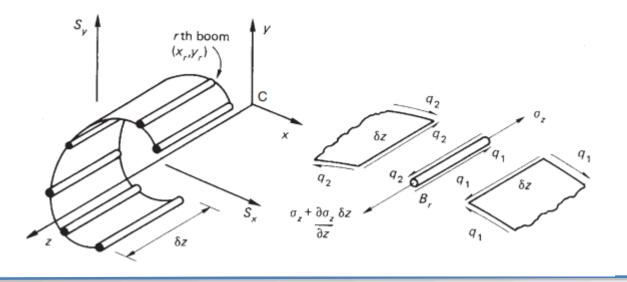


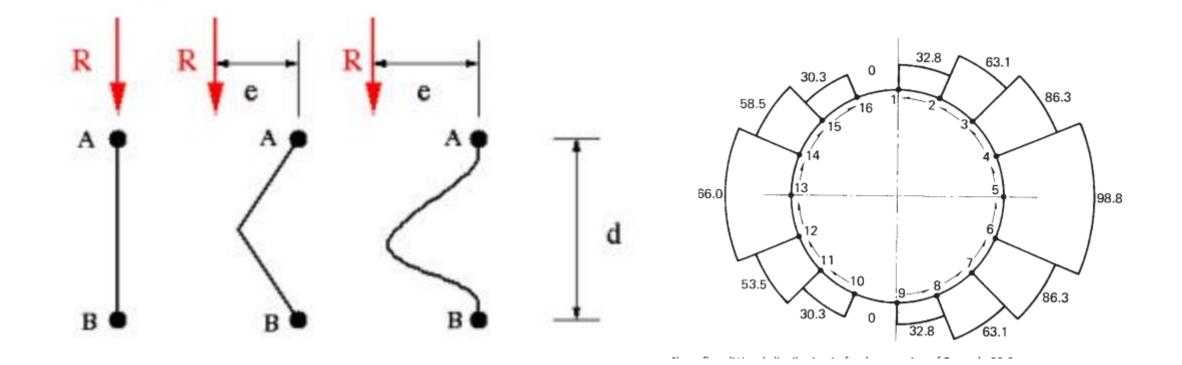






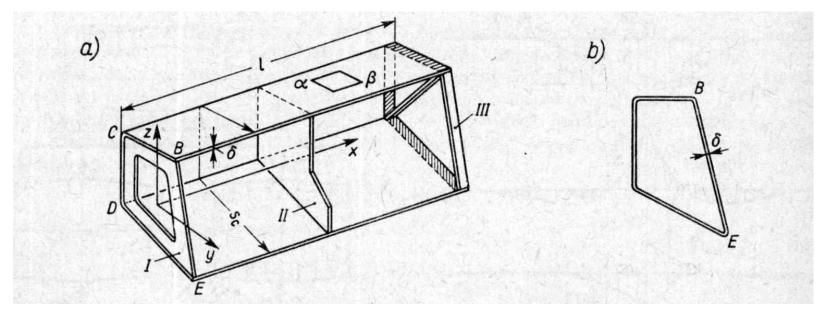






#### Assumptions:

- beam
- thin-walled
- CSRD



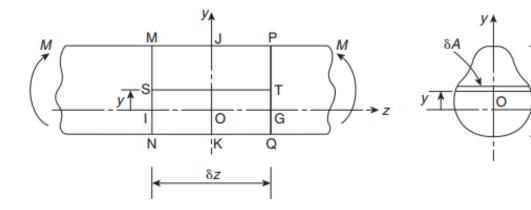
# Beams – bending (any section, any shape, any thickness)

#### Bending of open and closed, thin-walled beams

x Neutral

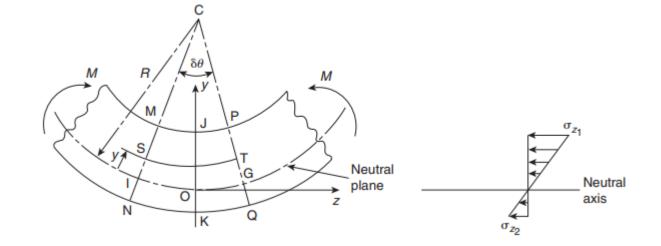
 $y_1$ 

 $y_2$ 

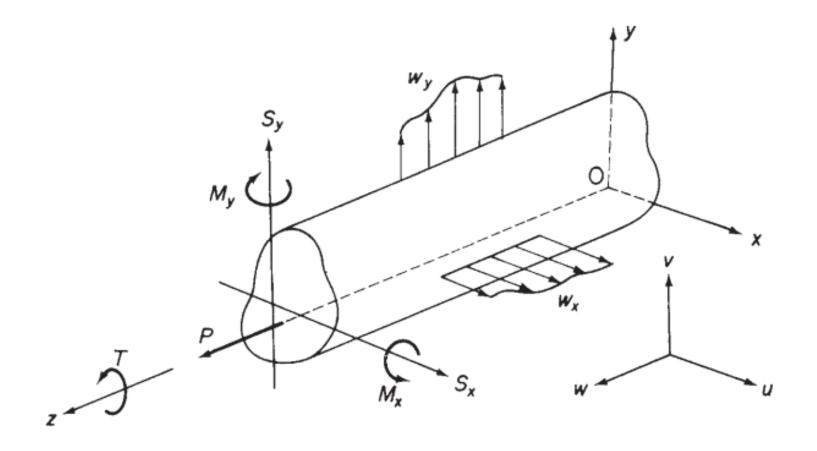




## Plane section Kirchhoff assumption



# Bending of open and closed, thin-walled beams SIGN convention

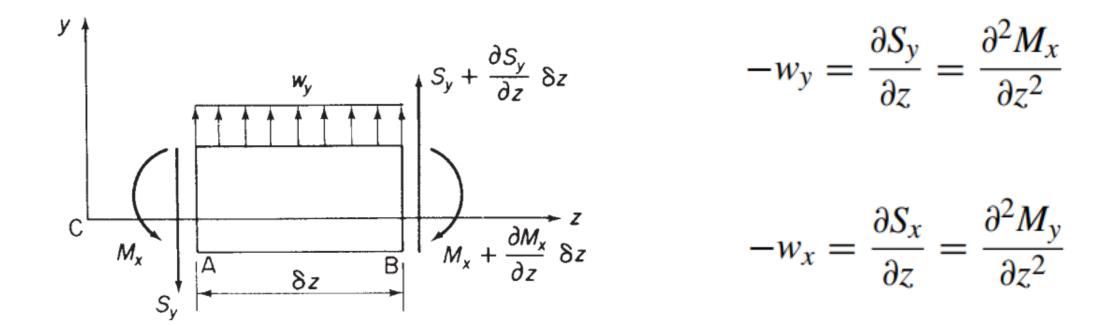


#### Bending of open and closed, thin-walled beams

$$\sigma_z = \left(\frac{M_y I_{xx} - M_x I_{xy}}{I_{xx} I_{yy} - I_{xy}^2}\right) x + \left(\frac{M_x I_{yy} - M_y I_{xy}}{I_{xx} I_{yy} - I_{xy}^2}\right) y$$

$$\sigma_z = \frac{M_x}{I_{xx}}y + \frac{M_y}{I_{yy}}x$$

#### Bending of open and closed, thin-walled beams

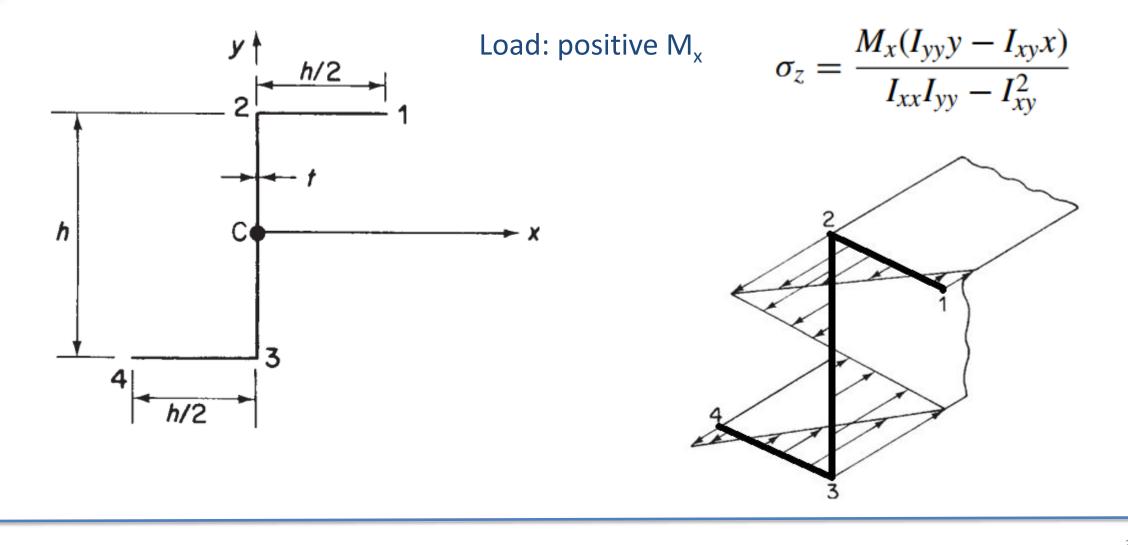


# Bending of open and closed, thin-walled beams deflections thru curvatures

$$\begin{cases} u'' \\ v'' \end{cases} = \frac{-1}{E(I_{xx}I_{yy} - I_{xy}^2)} \begin{bmatrix} -I_{xy} & I_{xx} \\ I_{yy} & -I_{xy} \end{bmatrix} \begin{cases} M_x \\ M_y \end{cases} \qquad \qquad M_x = -EI_{xy}u'' - EI_{xx}v'' \\ M_y = -EI_{yy}u'' - EI_{xy}v'' \end{cases}$$

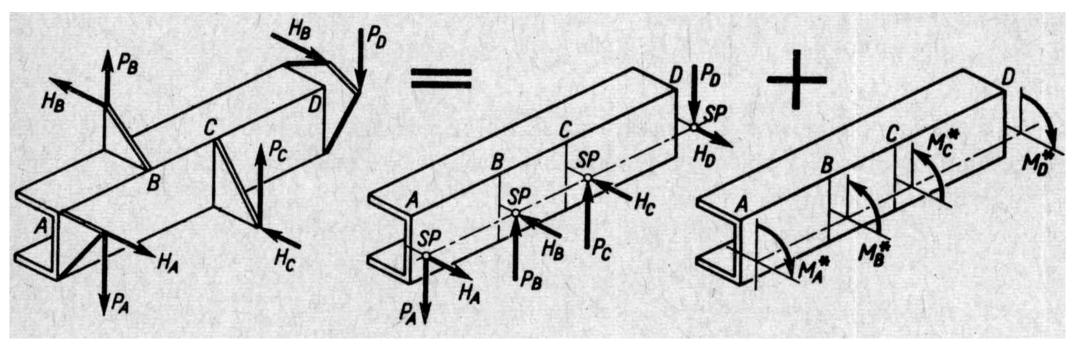
$$u'' = -\frac{M_y}{EI_{yy}}, \quad v'' = -\frac{M_x}{EI_{xx}}$$
  
 $M_y = -EI_{yy}u''$ 

#### Bending of open and closed, thin-walled beams

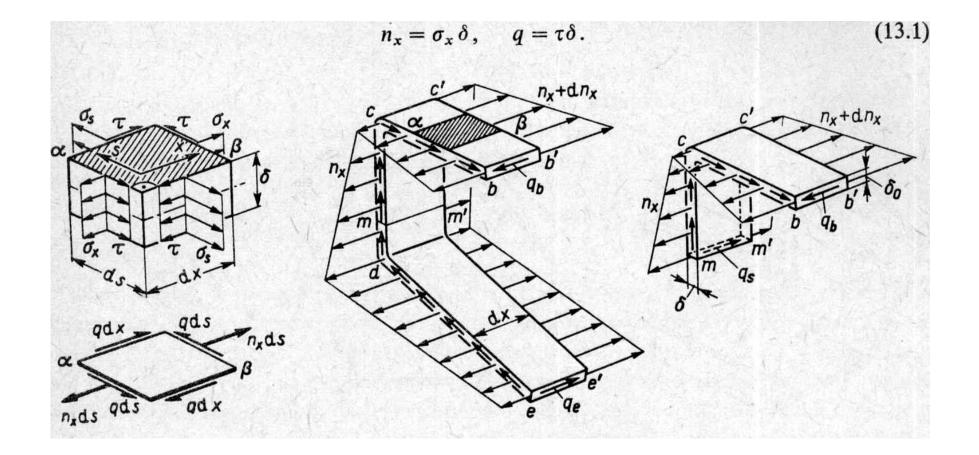


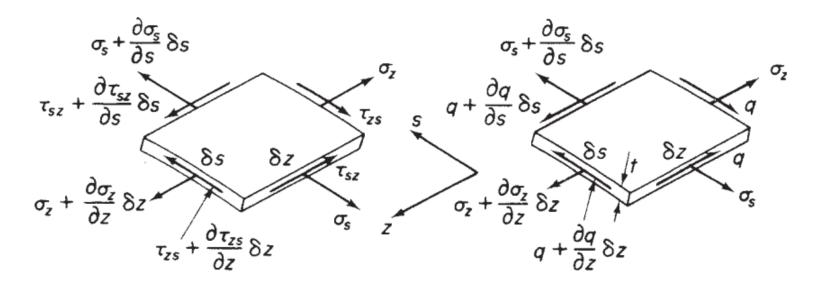
# **Thin-walled beams – bending / shear**

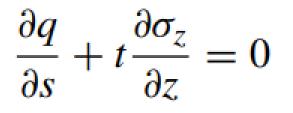
**open section** – later generalized for closed section

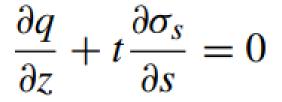


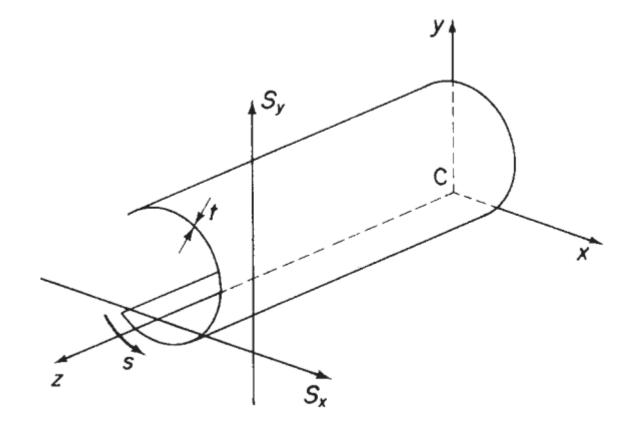
Arbitrary load split into bending/shear PLUS torsion No torsion = Shear center











#### No torsion = Shear center

$$\frac{\partial q}{\partial s} + t \frac{\partial \sigma_z}{\partial z} = 0 \qquad \qquad \frac{\partial \sigma_z}{\partial z} = \frac{\left[(\partial M_y/\partial z)I_{xx} - (\partial M_x/\partial z)I_{xy}\right]}{I_{xx}I_{yy} - I_{xy}^2} x + \frac{\left[(\partial M_x/\partial z)I_{yy} - (\partial M_y/\partial z)I_{xy}\right]}{I_{xx}I_{yy} - I_{xy}^2} y$$

$$\frac{\partial q}{\partial s} = -\frac{(S_x I_{xx} - S_y I_{xy})}{I_{xx} I_{yy} - I_{xy}^2} tx - \frac{(S_y I_{yy} - S_x I_{xy})}{I_{xx} I_{yy} - I_{xy}^2} ty$$

$$q_{s} = -\left(\frac{S_{x}I_{xx} - S_{y}I_{xy}}{I_{xx}I_{yy} - I_{xy}^{2}}\right) \int_{0}^{s} tx \, ds - \left(\frac{S_{y}I_{yy} - S_{x}I_{xy}}{I_{xx}I_{yy} - I_{xy}^{2}}\right) \int_{0}^{s} ty \, ds$$

$$q_s = -\frac{S_x}{I_{yy}} \int_0^s tx \, \mathrm{d}s - \frac{S_y}{I_{xx}} \int_0^s ty \, \mathrm{d}s$$